



STRUCTURE ALTERNATIVES EVALUATION REPORT

Region 2 Bridge Bundle Design Build Grant Project
Preliminary Design and Procurement Support Services

Structure J-15-G

(Region 2 – CO 9 MP 15.97)



Prepared for: Colorado Department of Transportation Region 2
5615 Wills Blvd.
Pueblo, CO 81008

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1. EXECUTIVE SUMMARY

1.1. PROJECT DESCRIPTION

The CDOT Region 2 Bridge Bundle Design Build Project consists of the replacement of seventeen (17) rural bridges on essential highway corridors in southeastern and central Colorado. The key corridors (US 350, US 24, CO 239 and CO 9) provide rural mobility, intra- and interstate commerce, movement of agricultural products and supplies, and access to tourist destinations. The 2 other bridges are Additionally Requested Elements (AREs) in the design build project. There is a total of nineteen (19) structures bundled under this project.

This design build project is partially funded by the USDOT FHWA Competitive Highway Bridge Program grant and funds from the Colorado Bridge Enterprise (14 structures, project number 23558). The 5 additional structures are funded solely by Colorado Bridge Enterprise (project number 23559). These projects are combined to form one design-build project.

The nineteen bridges identified to be included in the ‘Region 2 Bridge Bundle’ were selected based on similarities in the bridge conditions, risk factors, site characteristics, and probable replacement type, with the goal of achieving economy of scale. Seventeen of the bridges being replaced are at least 80 years old. Five of the bridges are Load Restricted limiting trucking routes through major sections of the US 24 and US 350 corridors. The bundle is comprised of nine timber bridges, four concrete box culverts, one corrugated metal pipe (CMP), four concrete I-beam bridges, and one I-beam bridge with corrugated metal deck.

1.2. PURPOSE OF THE REPORT

This report presents the findings of the preliminary level multidisciplinary investigation of the existing conditions of the given structure. The objective of this report is not to select a new structure type but to develop guidelines that will be addressed in the Design-Build documents and make recommendations based on the available information. All the information obtained in the survey, geotechnical investigation, hydrology and hydraulics, existing utilities, and environmental investigation is discussed in this report. The study evaluates feasible structure alternatives for the site and identifies all known constraints.

1.3. STRUCTURE SELECTION PROCESS

The following criteria for comparing and evaluating the structural alternatives is discussed below and will need to be considered during design-build processes:

- Hydraulic Opening Requirements
- Roadway alignments
- ROW Impacts
- Constructability
- Construction costs
- Maintenance
- Durability
- Traffic Control

The recommendations of the report are based on the overall consideration of all these elements as appropriate to this site and bridge.

1.4. STRUCTURE RECOMMENDATIONS

Based on the subsequent discussion, the recommended proposed structure that provides the required hydraulic opening and meets other design criteria is a two-cell 20.0 ft x 6.0 ft concrete box culvert, similar to CDOT M-601-2 standard. The proposed length of the structure will be 64 ft. This length of proposed structure construction must accommodate two 12.0 ft lanes of traffic with 8.0 ft shoulders.

The contractor may select a different structure type based on their investigation, meeting the criteria described in this report.

2. SITE DESCRIPTION AND DESIGN FEATURES

2.1. EXISTING STRUCTURE

Existing structure consists of two-84" galvanized CMP pipes, built in 1971 to allow for the Mack Gulch to cross under State Highway (CO) 9. The structure is located on a 45-degree skew, with an out-to-out length of 60.0ft. This structure has no special end treatments. The pipes are on a 4.36% vertical slope with approximately 18 in of cover

The bridge is located on CO 9, 6.2 miles south of Guffy, Colorado, at milepost 15.97, 14 miles north of the CO 9 and US 50 junction. Table 1 summarizes bridge information.

The replacement of Structure J-15-G is warranted due to the age and deteriorating conditions of the CMP pipes.

Pipe 1: Invert has heavy corrosion. There is some flaking, swelling, and section loss. Bulging and dents are present at multiple locations. There is 1.0 in vertical separation between joints.

Pipe 2: Heavy corrosion is present at both the inlet and outlet of the pipe. This includes flaking, swelling, and section loss. Pipe is bent at outlet.

National Bridge Structure Number	J-15-G
Year Built	1971
Construction Type	(2) 84" CMP pipes
Condition Rating	Poor
Load Restricted	No
Structure Length	NA
Structure Width	60 feet
Number of spans	2
Water Crossing	Mack Gulch
ADT	1300
Percent Commercial Traffic	7.4%

Table 1 – J-15-G Summary Information



Picture 1 – J-15-G



Picture 2 – Inside of pipe



Picture 3 – Inlet damage

2.2. RIGHT OF WAY IMPACT

The existing Right-Of-Way (ROW) is located approximately 50.0 ft from the centerline of the CO 9 on each side of the structure. Any alternative selected by a design-build team shall not impact the existing ROW. No permanent ROW acquisitions are planned on either side of CO 9. Temporary construction easements may be required during construction. A retaining wall will need to be constructed on the north side of the structure to contain the side slope of the proposed roadway section within the ROW.

2.3. TRAFFIC DETOUR OR SHOOFLY

As stated by the CDOT grant application, the roadway shall not be closed for construction. Two other alternatives were investigated:

1. Phasing the constructions to keep one lane open. To meet all typical CDOT roadway phased construction criteria, this alternative can be constructed keeping half of the existing culvert open to traffic.
2. Building a one- or two-lane shoofly on either side of the existing bridge with a temporary pipe placed for drainage. The impacts to wetlands make this option less desirable

Alternative 1 (phased construction) was identified as the preferred traffic alternative for this structure. More information on traffic detour options can be found in the Traffic Design Memorandum for this structure.

2.4. UTILITIES

Stanley subcontracted with Lamb-Star Engineering to provide utility location services in the vicinity of the structure. Based on their investigation, no known utilities are located in the vicinity of the structure.

2.5. GEOTECHNICAL SUMMARY

Stanley subcontracted with Yeh and Associates, Inc. to perform the geotechnical investigation of all bridges in this project. Full Preliminary Geotechnical Study is provided in the Appendix D.

Two bridge borings, J-15-G-B-1 and J-15-G-B-2, were drilled by Yeh in the vicinity of the existing CMP pipes, and two pavement borings, J-15-G-P-1 and J-15-G-P-2, were drilled along the existing pavement approximately 250 feet from the CMP pipes.

The bridge borings encountered silty sand overlying diorite and granite bedrock. Table 2 provides a summary of the bedrock and groundwater conditions for the bridge borings. The surface elevations, approximate bedrock depths/elevations, and approximate groundwater depths/elevations are presented to the nearest 0.5 feet. The groundwater depths and elevations are based on observations during drilling.

Boring ID	Ground Surface Elevation at Time of Drilling (feet)	Approx. Depth to Top of Competent Bedrock (feet)	Approx. Elevation to Top of Competent Bedrock (feet)	Approx. Groundwater Depth (feet)	Approx. Groundwater Elevation (feet)
J-15-G- B-1	8007.0	19.5	7987.5	9.0	7998.0
J-15-G- B-2	8008.5	25.0	7983.5	11.0	7997.5

Table 2 - Summary of Bedrock and Groundwater Conditions

If CBC structure is selected, then the structure will be founded on shallow mat foundation. Wingwalls for the CBC structure will be founded on shallow strip foundations.

2.6. HYDRAULICS SUMMARY

Structure J-15-G conveys flow from Mack Gulch under CO 9. The design flow rate is the 25-year storm event which produces 1341 cfs upstream of the structure. The proposed replacement must not allow more than 0.5 feet of rise in the 100-year water surface elevation per state law. An SRH-2D model was developed at this location. The proposed model indicates that both a two-cell 20 ft x 6 ft concrete box culvert and four-84" diameter RCP pipes will carry the flows without causing a rise of more than 0.5 feet.

There is no freeboard requirement for both the proposed box culvert option, and RCP option; however, the culvert must meet Headwater Depth to Structure Depth ratio (HW/D) of 1.5 per the CDOT Drainage Design Manual. The HW/D is 0.9 for the proposed RBC and 1.08 for the proposed RCP.

A Preliminary Hydraulic Report has been completed and can provide more information about the existing and proposed hydraulics conditions.

2.7. ENVIRONMENTAL CONCERNS

Based on field investigation performed by Stanley Consultants Environmental team, the project has the potential to impact 0.28 acres of USACE jurisdictional tributaries. The project has no potential to impact species listed under the Federal Endangered Species Act (FECA) or Colorado Parks and Wildlife (CPW), but may impact nine BLM sensitive species, with potential for Migratory Bird Treaty Act (MBTA) species and bats to occur.

A permit may be required under section 404 of the Clean Water Act if any impacts come to a USACE regulated surface water.

Coordination with BLM will likely be required, as well as with CPW, as a SB 40 wildlife certification may be required. Clearance of MBTA species may be required prior to construction.

Refer to Desktop Study and wetland reports for additional information.

2.8. ROADWAY FEATURES

2.8.1. Cross Section

Existing CO 9 is a 2-lane roadway with two-way traffic. Both lanes are 11.0 ft wide with 1.0 ft shoulders. There is no existing guardrail on either side of the road.

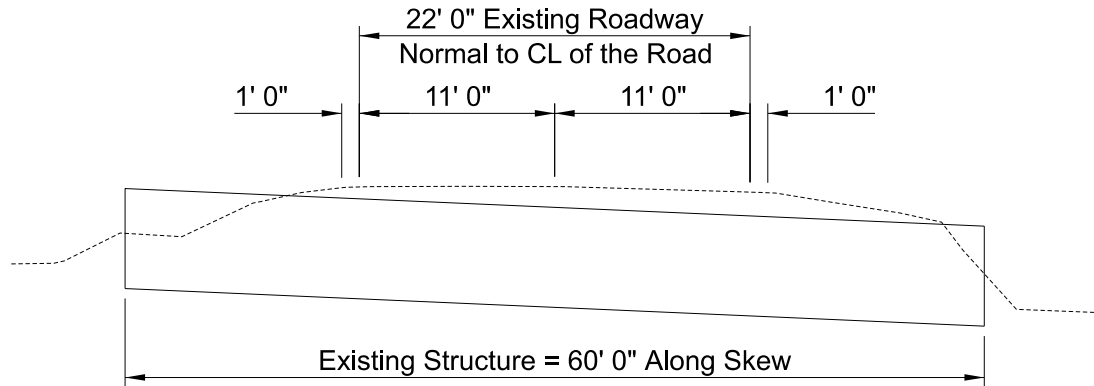


Figure 1 - Existing Roadway Section

The proposed roadway section width is based on the traffic volumes and the requirements of the current CDOT Roadway Design Guide. Lane width is expected to be 12.0 ft in each direction with 8.0 ft shoulders. Total required roadway width over proposed structure is 40.0 ft. Additional roadway width is needed for phased construction and is discussed in the section 4.7 Construction Phasing.

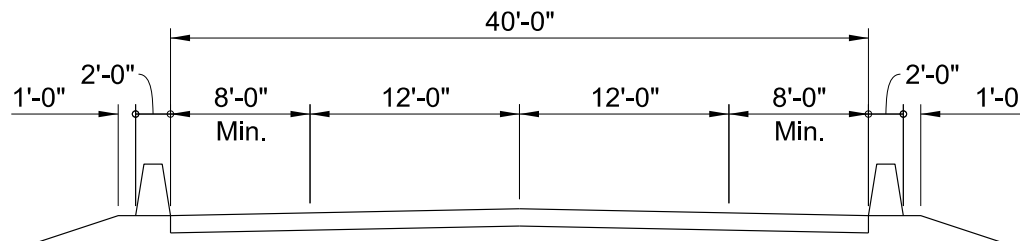


Figure 2 - Proposed Roadway Section

2.8.2. Vertical Alignment

The proposed vertical profile of CO 9 will raise the existing profile by an average of 4 ft in order to construct the proposed structure under the roadway with an adequate roadway section over the structure and to prevent overtopping of the roadway. The proposed tangent slope over the structure is 2.99 %.

2.8.3. Horizontal Alignment

The horizontal alignment of the existing CMP pipes has approximately 36-degree skew. The CMP pipes are located on a horizontal tangent between two curves. It is understood that the proposed structure will be constructed in the same location as the existing with no change to the CO 9 horizontal alignment.

3. STRUCTURAL DESIGN CRITERIA

3.1. DESIGN SPECIFICATIONS

- AASHTO LRFD Bridge Design Specifications, 9th Edition
- CDOT LRFD Bridge Design Manual
- CDOT Bridge Rating Manual
- CDOT Bridge Detail Manual

3.2. CONSTRUCTION SPECIFICATIONS

Colorado Department of Transportation Standard Specifications for Road and Bridge Construction, 2019.

3.3. LOADING

Live Loads: HL-93 Design Truck or Tandem, Design Lane Load, Colorado Permit Vehicle

Bridge Barrier: Bridge Rail Type 10MASH or Bridge Rail Type 9 per the CDOT standards.

Future Wearing Surface: 36.67 lbs per square foot (3 in minimum)

Utilities: per plan details if required at final design

Collision Load: the substructure will not require collision loading design. In cases where Bridge Rail is attached to the structure, the effects of vehicular collision on the barrier must be considered in accordance with AASHTO.

Earthquake Load: The structure is located within Seismic Zone 1 and must meet the AASHTO connection and detailing requirements.

Stream Forces and Scour Effects: stream force effects must be evaluated during final design when applicable. Possible cases include stream forces on the substructure and superstructure in addition to buoyancy from overtopping. Evaluation from scour will be performed per the CDOT Bridge Design Manual and AASHTO.

4. STRUCTURE SELECTION

4.1. SELECTION CRITERIA

The goal of this report is to identify which structural alternatives best meet the project requirements. The following criteria were established as a basis for evaluating the suitability of each structure type: hydraulic opening, constructability, construction cost, maintenance & durability, ROW and roadway impacts. The discussion below expands on these factors as it pertains to each alternative. Summary of Structure Alternatives Evaluation Table can be found at the end of Section 4.

4.2. REHABILITATION ALTERNATIVES

Rehabilitation of J-15-G will not be performed due to the age and deteriorating condition of the CMP pipes. Built in 1971, these CMP pipes have been in service for 50 years and are showing significant signs of deterioration and aging that are inconsistent with practical and cost-effective rehabilitation.

4.3. STRUCTURE LAYOUT ALTERNATIVES

Layout of the proposed structure is controlled by the width of the proposed roadway section, stream geometry, hydraulic opening requirements, proximity of ROW line, and the position of the existing CMP pipes.

The replacement structure will require a headwater to depth ratio of less than 1.5. For this location, all proposed alternatives satisfy this requirement.

The horizontal alignment of the proposed structure will have 36-degree skew to match the existing CMP pipes and channel geometry.

4.4. SUPERSTRUCTURE ALTERNATIVES

4.4.1. Concrete Box Culvert Alternative

Concrete box culverts are a cost-effective solution in both short- and long-term due to the ease of construction and maintenance. The benefit of this structure type is that the culverts can be cast-in-place (CIP) or precast off-site and transported to the site for placement to streamline the construction process. In addition, CBC size can be selected from CDOT M&S Standards that cover wide array of single-cell and multi-cell culvert sizes.

For J-15-G a two-cell 20 ft x 6 ft box culvert is required. The box is estimated to have a total height of approximately 8 ft 10 in. Fill height will vary from 6.0 ft to 1.0 ft. The box can be constructed as CIP or precast. The south headwall of the proposed CBC will be placed in-line with the proposed retaining wall located on the south-east side of the structure. The north headwall will be placed as required to accommodate phased construction. The total proposed CBC length is 64.0 ft.

Wingwalls will be provided on 3 corners of the CBC. Wingwalls will be per CDOT M-601-20 standard.

The CBC will require a riprap apron on the downstream side of the structure as an energy dissipation countermeasure.

4.4.2. Reinforced Concrete Pipe Alternatives

Reinforced Concrete Pipes provide a lot of the same benefits of concrete box culverts with the exception that RCP pipes have concerns with long term maintenance. For J-15-G, four (4) 84 in diameter RCP pipes will be required. Similarly to the CBC alternative, RCP alternative will require headwalls and wingwalls. The south headwall of the proposed RCP will be placed in-line with the proposed retaining wall located on the south-east side of the structure. The north

headwall will be placed as required to accommodate phased construction. The total proposed RCP length is 64.0 ft.

Wingwalls will be provided on 3 corners of the RCP. Wingwalls will be per CDOT M-601-20 standard.

The RCP will also require a riprap apron on the downstream side of the structure as an energy dissipation countermeasure.

4.5. SUBSTRUCTURE ALTERNATIVES

The replacement structure will consist of either a Reinforced Concrete Pipe (RCP) structure or a concrete box culvert structure (CBC). If an RCP structure is selected, no wingwalls, headwall, or toewall will be required for the structure. If CBC structure is selected, then the structure and its wingwalls will be founded on shallow mat foundation. Only three Wing walls will be needed for the CBC as the South East Corner of the headwall will tie into the retaining wall running parallel to the road.

The design and construction of the foundation elements should comply with all applicable requirements and guidelines listed in AASHTO (2020) and the CDOT Standard Specifications (CDOT 2019).

4.6. ACCELERATED BRIDGE CONSTRUCTION (ABC)

CDOT has developed an Accelerated Bridge Construction (ABC) decision making process. The intent of this process is to apply some form of ABC on most projects. Design-build team is encouraged to use these recourses to evaluate cost efficiency of implementing ABC design.

4.7. CONSTRUCTION PHASING

As discussed in Section 2.3 building a shoofly at this location would have additional impacts to the existing wetlands. Phased construction is feasible and recommended.

Based on the CDOT Roadway requirements a minimum required roadway configuration for each phase of the construction must consist of an 11.0 ft lane, 2.0 ft shoulder on each side, 2.0 ft wide temporary concrete barrier, 1.0 ft minimum work zone buffer with pinned barrier and 2.0 ft minimum work zone buffer with non-pinned barrier. To accommodate these requirements, all proposed alternatives will require some amount of overbuild, (compared to the approaching roadway section). Figure below shows required phasing configuration for both alternatives. More information on phased construction can be found in the traffic Design Memorandum for this structure.

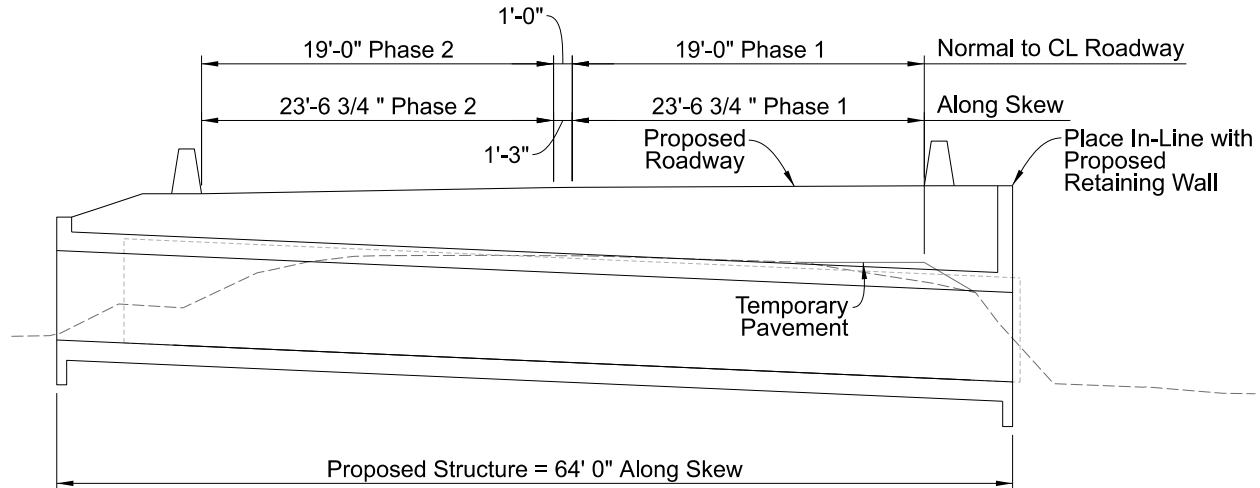


Figure 3 - Construction Phasing (CBC shown, RCP similar)

4.8. CONSTRUCTABILITY

Both alternatives are proposed to be constructed in phases. Shoring will be required to construct any of the proposed alternatives. The option is available to use precast for both the CBC and RCP alternatives, which would reduce construction time.

4.9. MAINTENANCE AND DURABILITY

Typical CDOT specified materials and construction methods must be used for the construction of the proposed structure. Following accepted current practice in designing and constructing the structure will be required to meet the required 100-year service life with minimal required maintenance. Both the proposed CBC and RCP alternatives may require routine cleaning; however CBC are easier to clean and inspect.

4.10. CORROSIVE RESISTANCE

Epoxy coated reinforcing must be used for all reinforced concrete elements. A waterproofing membrane and stone matrix asphalt will be used on top of the CBC to prevent water and salt intrusion.

4.11. CONSTRUCTION COST

Construction costs are one of the most important factors in the structure type selections. Preliminary construction cost estimates are prepared for all selected structure alternatives to be compared as discussed above. High level construction cost for each structure type is summarized in the table below. Detailed calculations of the cost can be found in the Appendix C of this report. Individual items cost was obtained from recent CDOT Cost Data Books. A 30% contingency multiplier was used in cost calculations.

Alternative	Construction Cost	Area	Cost (\$/sf)	Cost Rating
CBC	\$ 696,000.00	2752 sf	\$ 253	1.0
RCP	\$ 615,000.00	3136 sf	\$ 196	1.1

Table 3 - Construction Cost Summary

4.12. CONCLUSIONS AND RECOMMENDATIONS

Table 4 provides a summary of feasible alternatives evaluation based on the established selection criteria.

Criteria	CBC	RCP
Hydraulic Opening	Satisfies the requirements	Satisfies the requirements
Constructability	No expected constructability issues. Can be precast to streamline construction. Phased construction requires shoring.	No expected constructability issues. Can be precast to streamline construction. Phased construction requires shoring.
Construction Cost Rating	1.0	1.1
Maintenance & Durability	May require routine cleaning. Preferred for long-term maintenance	May require routine cleaning. Harder to clean and inspect
ROW and Roadway Impacts	Retaining wall will be constructed to prevent ROW impacts	Retaining wall will be constructed to prevent ROW impacts.

Table 4 - Summary of Structure Alternatives Evaluation

Based on the criteria discussed above, the CBC alternative is recommended to replace existing structure J-15-G. Even though it is slightly more expensive solutions, the comparative ease of the long-term maintenance warrants this choice. The contractor may select a different structure type based on their investigations, meeting the criteria described in this report. See Appendix A for the selected General Layout and Typical Section.

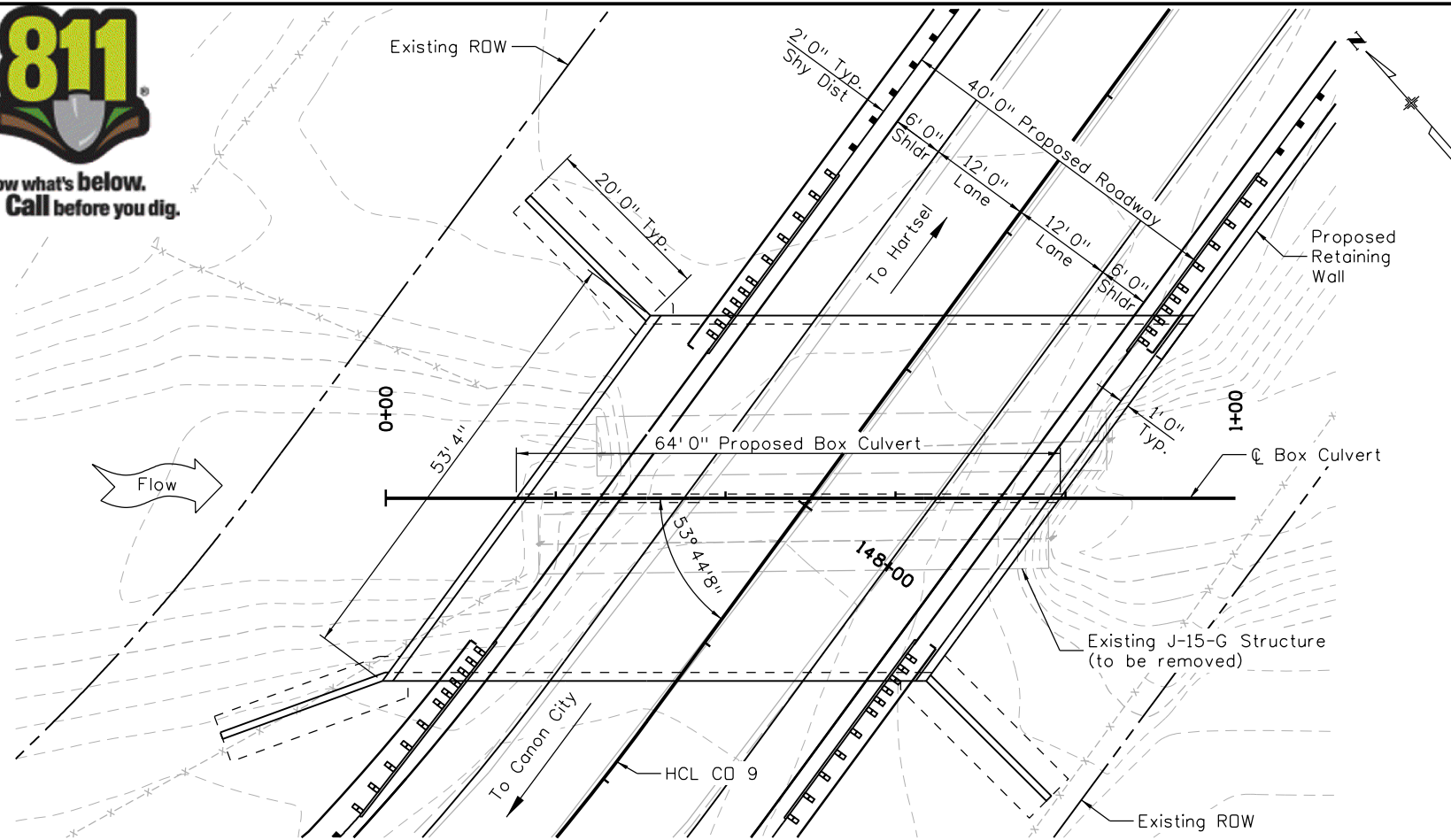
APPENDIX A

General Layout and Typical Section

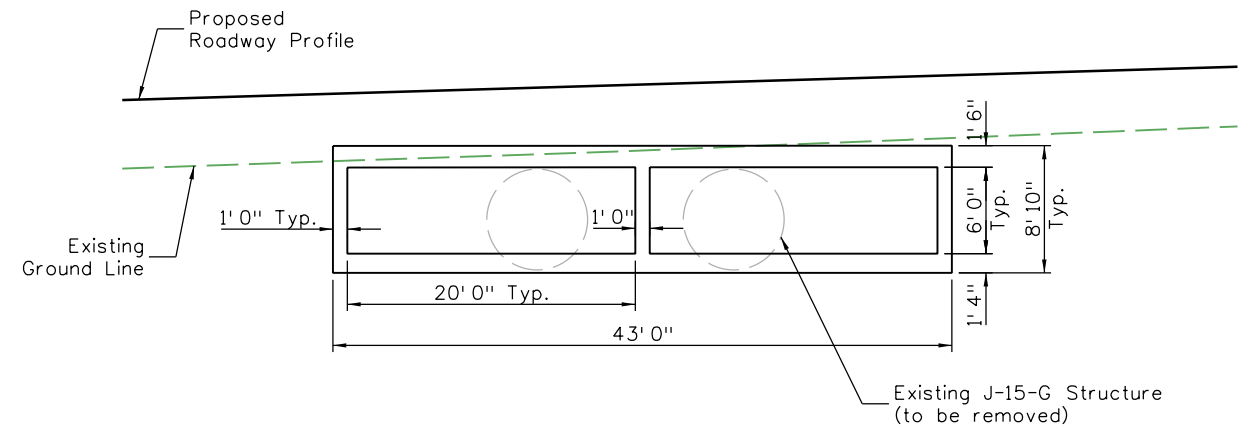
9365 11:08:20 AM pw:\SCI-PW\INTEG-2.stanleygroup.com\Datasearch-1\Documents\Colorado_Department_of_Transportation\29715 - CDDT R2 Bridge Bundle\11-CADD\Worksets\29175\DCNs\Bridges\General Layout G-12-C-SH9 MP 71.44.dgn



Know what's below.
Call before you dig.



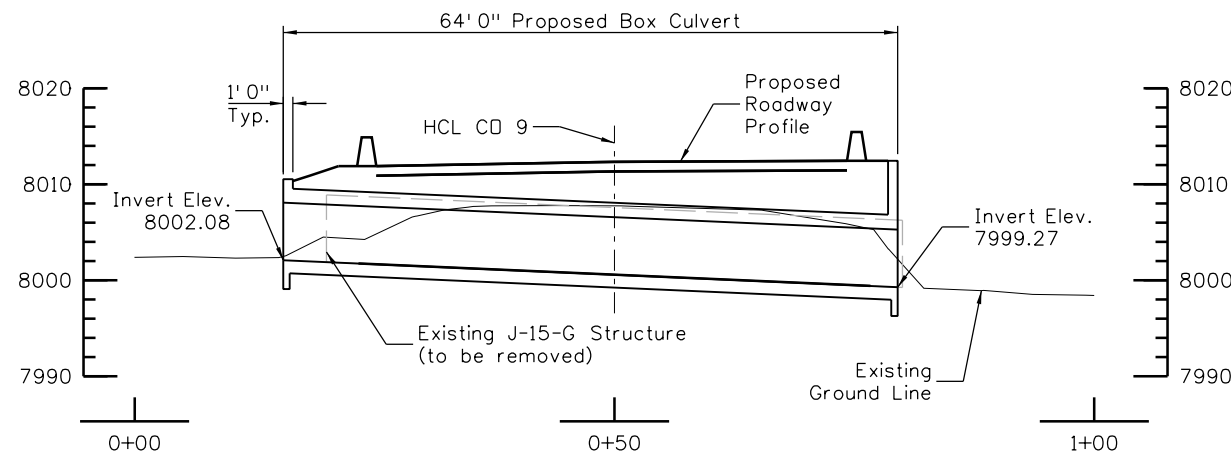
PLAN



SECTION
(Taken Normal to ϕ Structure)

Notes:

- Dimensions are based on recent survey and existing as-built drawings.



PROFILE
(Taken Along ϕ Structure)

Print Date: 1/22/2021 File Name:

23558STR_General Layout J-15-G_SH9 MP15.970.dgn

Horiz. Scale: Vert. Scale: As Noted

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8000 South Chester St.
Suite 500
Centennial, CO 80112
Phone: 303-799-6806

Sheet Revisions

Date:	Comments	Init.

Colorado Department of Transportation



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Phone: 719-546-5753
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Region 2

JLS

As Constructed

No Revisions:

Revised:

Void:

REGION 2 BRIDGE BUNDLE
US 9 OVER MACK GULCH
ROADWAY PLAN AND PROFILE

Designer: I. Pushkarova Structure No. J-15-G

Detailer: I. Pushkarova M.P. CD 9 M.P. 15.970

Sheet Subset: STR Subset Sheets: 1 of 1

Project No./Code

Sheet Number

APPENDIX B

Structure Selection Report Checklist

Structure Selection Report QA Checklist

This checklist is to serve as a general guideline for structure selection process. It is to be filled out by the project Engineer of Record or designee to indicate all items that are to be discussed in the Structure Selection Report. This checklist is to be included as an appendix to the Structure Selection Report and must be signed by Staff Bridge Unit Leader or designee prior to submittal of FIR documents to the Region.

Project Name _____

Project Location _____

Project Number _____ Subaccount _____

Structure Number(s) _____

Engineer of Record _____ Date _____

Cover Sheet

- Name of the Project and Site Address
- Structure(s) Number
- Property Owner Name and Contact Information
- Report Preparer Name and Contact Information
- Seal and Signature of the Designer
- Submittal and Revision Dates as Applicable

Executive Summary

- Project Description
- Purpose of the Report
- Structure Selection Process
- Structure Recommendations

Site Description and Design Features

- Existing Structures
- ROW Impact
- Traffic Detour
- Utilities
- Geotechnical Summary
- Hydraulics Summary
- Environmental Concerns
- Roadway Design Features
 - Cross Section
 - Vertical Alignment
 - Horizontal Alignment

Structural Design Criteria

- Design Specifications
- Construction Specifications
- Loading
 - Collision Load
 - Earthquake Load
- Software to be used by the Designer
- Software to be used by the Independent Design Checker

Structure Selection

- Selection Criteria
- Rehabilitation Alternatives
- Structure Layout Alternatives:
 - Vertical Clearances
 - Horizontal Clearances
 - Deflection
 - Skew

- Superstructure Alternatives:
 - Concrete Girder Alternatives * CBC Alternative
 - Steel Girder Alternatives * RCP Alternative
 - Span Configurations
- Substructure Alternatives:
 - Abutment Alternatives (GRS, Integral, Semi-integral, etc.)
 - Pier Alternatives
- Wall Alternatives
- Construction Phasing
- Possible Future Widening
- Use of Existing Bridge in Phasing / Partial Configuration
- ABC Design
- Constructability
- Aesthetic Design
- Maintenance and Durability
- Corrosive Resistance
- Load Testing Requirements
- Use of Lightweight Concrete
- Construction Cost
- Life Cycle Cost Analysis

Other

Figures and Appendices

- Vicinity Map
- Alternative Typical Sections
- General Layout of the Selected Structure
- Summary of Structure Type Evaluation Table
- Summary of Quantities and Cost Estimate Tables
- Inspection Report
- Hydraulics Investigation Results
- Geotechnical Investigation Results

Recommendations

If you need more space, use an additional sheet(s) of paper.

List of Variances

If you need more space, use an additional sheet(s) of paper.

CDOT Staff Bridge Quality Assurance Sign-off

By signing this checklist Staff Bridge Unit Leader or designee acknowledges approval of the Structure Selection Report findings, recommendations, and all design deviations from the CDOT Structural Standards and design criteria.

Print Name Signature Date

APPENDIX C

Construction Cost Estimate

Project No.: CDOT #23558 (Stanley #29715)
 Project Name: Region 2 Bridge Bundle Design Build Grant Project
 Subject: Quantity Calculations - J-15-G CBC Alternative
 Client: CDOT Region 2

Date: 2/16/2021

CBC Alternative					
Contract Item No.	Item Description	Unit	Estimated Unit Cost	TOTAL	
				Approx Quantities	Estimated Total Cost
202-00400	Removal of Bridge	EACH	\$ 50,000.00	1	\$ 50,000
206-00000	Structure Excavation	CY	\$ 20.00	936	\$ 18,729
206-00100	Structure Backfill (Class 1)	CY	\$ 35.00	680	\$ 23,783
206-01750	Shoring	L S	\$ 15,000.00	1	\$ 15,000
515-00120	Waterproofing (Membrane)	SY	\$ 22.50	342	\$ 7,705
601-04550	Concrete Class G	CY	\$ 900.00	376	\$ 338,104
601-40300	Structural Concrete Coating	SY	\$ 14.00	129	\$ 1,803
602-00020	Reinforcing Steel (Epoxy Coated)	LB	\$ 1.50	53625	\$ 80,437
Subtotal of accounted construction items =>					\$ 535,561
Contingency Multiplier =>					30%
Subtotal of construction items =>					\$ 696,229
Deck area (SF) =>					2752
Cost per SF =>					\$ 253

Project No.: CDOT #23558 (Stanley #29715)
 Project Name: Region 2 Bridge Bundle Design Build Grant Project
 Subject: Quantity Calculations - J-15-G RCP Alternative
 Client: CDOT Region 2

Date: 2/16/2021

RCP Alternative					
Contract Item No.	Item Description	Unit	Estimated Unit Cost	TOTAL	
				Approx Quantities	Estimated Total Cost
202-00400	Removal of Bridge	EACH	\$ 50,000.00	1	\$ 50,000
206-00000	Structure Excavation	CY	\$ 20.00	987	\$ 19,740
206-00100	Structure Backfill (Class 1)	CY	\$ 35.00	890	\$ 31,137
206-01750	Shoring	L S	\$ 15,000.00	1	\$ 15,000
601-04550	Concrete Class G	CY	\$ 900.00	103	\$ 92,434
601-40300	Structural Concrete Coating	SY	\$ 14.00	138	\$ 1,931
602-00020	Reinforcing Steel (Epoxy Coated)	LB	\$ 1.50	21445	\$ 32,167
603-01845	84 Inch Reinforced Concrete Pipe (Complete In Place)	LF	\$ 900.00	256	\$ 230,400
Subtotal of accounted construction items =>					\$ 472,809
Contingency Multiplier =>					30%
Subtotal of construction items =>					\$ 614,652
Deck area (SF) =>					3136
Cost per SF =>					\$ 196

APPENDIX D

Geotechnical Report



February 11, 2021

Project No. 220-063

Mr. Ron Gibson, P.E.
Stanley Consultants
8000 South Chester Street, Suite 500
Centennial, Colorado 80112

**Subject: Preliminary Geotechnical Study
Structure J-15-G
23558/23559 Region 2 Bridge Bundle
CDOT Region 2, Colorado**

Dear Mr. Gibson:

This memorandum presents the results of Yeh and Associates, Inc.'s (Yeh) preliminary geotechnical engineering study for the proposed replacement of Structure J-15-G as part of the CDOT Region 2 Bridge Bundle Design-Build Project.

The CDOT Region 2 Bridge Bundle Design-Build Project consists of the replacement of a total of 19 structures bundled together as a single project. These structures are rural bridges on essential highway corridors (US 350, US 24, CO 239, and CO 9) in southeastern and central Colorado. These key corridors provide rural mobility, intra- and interstate commerce, movement of agricultural products and supplies, and access to tourist destinations. The design-build project consists of 17 bridges and two Additionally Requested Elements (ARE) structures.

This design-build project is jointly funded by the USDOT FHWA Competitive Highway Bridge Program grant (14 structures, Project No. 23558) and the Colorado Bridge Enterprise (five structures, Project No. 23559). These projects are combined to form one design-build project. The two ARE structures are part of the five bridges funded by the Colorado Bridge Enterprise.

The 19 bridges identified to be included in the Region 2 Bridge Bundle were selected based on similarities in the bridge conditions, risk factors, site characteristics, and probable replacement type, with the goal of achieving economy of scale. Seventeen of the bridges being replaced are at least 80 years old. Five of the bridges are load-restricted, limiting trucking routes through major sections of the US 24 and US 350 corridors. The bundle includes nine timber bridges, four concrete box culverts (CBC), one corrugated metal pipe (CMP), four concrete I-beam bridges, and one I-beam bridge with corrugated metal deck.

1 PROJECT UNDERSTANDING

Structure J-15-G is part of the Region 2 Bridge Bundle project that will be delivered as a design-build project. Our preliminary geotechnical study was completed to support the 30% design level that will be included in the design-build bid package. We understand the existing structure is a corrugated metal pipe (CMP) and will be replaced with either a CBC or a bridge structure. The new structure will be constructed along the current

roadway alignment and existing roadway grade will be maintained. No significant cut or fills are required for construction of the proposed replacement structure.

2 SUBSURFACE CONDITIONS

Two bridge borings, J-15-G B-1 and J-15-G B-2 were drilled by Yeh in the vicinity of the existing CMP, and two pavement borings, J-15-G P-1, and J-15-G P-2, were drilled along the existing pavement approximately 250 feet from the CMP. The approximate boring locations are shown on the engineering geology sheet in Appendix A. The legend and boring logs are included in Appendix B. Laboratory test results are provided in Appendix C and are shown on the boring logs.

The bridge borings encountered silty sand overlying diorite and granite bedrock. Table 1 provides a summary of the bedrock and groundwater conditions for the bridge borings. The surface elevations, approximate bedrock depths/elevations, and approximate groundwater depths/elevations are presented to the nearest 0.5 feet. The groundwater depths and elevations are based on observations during drilling.

Table 1. Summary of Bedrock and Groundwater Conditions

Boring ID	Location ¹ (Northing, Easting)	Ground Surface Elevation at Time of Drilling ¹ (feet)	Approx. Depth to Top of Competent Bedrock ¹ (feet)	Approx. Elevation to Top of Competent Bedrock ¹ (feet)	Approx. Groundwater Depth ^{1,2} (feet)	Approx. Groundwater Elevation ^{1,2} (feet)
J-15-G B-1	305328.2, 1005677.1	8007.0	19.5	7987.5	9.0	7998.0
J-15-G B-2	305328.1, 1005630.0	8008.5	25.0	7983.5	11.0	7997.5

Notes:

(1) Surface elevations, approximate bedrock depths/elevations, and approximate groundwater depths/elevations are presented to the nearest 0.5 feet. Location and elevation are provided by project surveyor.

(2) Groundwater depths and elevations are based on observations during drilling.

3 BRIDGE FOUNDATION RECOMMENDATIONS

We understand that the replacement structure will consist of either a new bridge structure or a CBC. If a bridge structure is selected, then the abutments and piers will be supported on driven H-piles or drilled shafts. If a CBC structure is selected, then the structure will be founded on a shallow mat foundation. Wing walls for the bridge and CBC structures will be founded on shallow strip foundations.

Based on the subsurface conditions encountered during our preliminary study, our engineering analysis, and our experience with similar projects, it is our opinion that driven H-pile and drilled shaft foundations are suitable for support of the bridge structure. Shallow foundations are suitable for support of the CBC and wing wall structures. Recommendations for the drilled shafts are presented in Section 3.2, driven H-pile recommendations are provided in Section 3.3, and CBC foundation recommendations are presented in Section 3.4.

The soil and bedrock properties were estimated from penetration resistance, material descriptions, and laboratory data. The design and construction of the foundation elements should comply with all applicable requirements and guidelines listed in AASHTO (2020) and the CDOT Standard Specifications (CDOT 2019).



3.1 Shallow Foundation Recommendations

Based on the depth to competent bedrock and the anticipated loading requirements, it is our opinion that shallow foundations are not suitable to support the bridge abutments. Bedrock is anticipated about 19 to 25 feet below the existing pavement surface.

3.2 Drilled Shaft Recommendations

3.2.1 Drilled Shaft Nominal Axial Resistance

The estimated bearing resistance should be developed from the side and tip resistance in the underlying competent bedrock. The resistance from the overburden soil should be neglected. The design approach in Abu-Hejleh et al. (2003) provides recommendations for the use of an updated Colorado SPT-based (UCSB) design method. In this design method, the nominal side and tip resistance of a drilled shaft in the bedrock is proportional to the driven sampler penetration resistance. This approach was generally used to estimate the axial resistance in the bedrock where UCS test results were unavailable. Based on local practice, the modified California penetration resistance is considered to be equivalent to SPT penetration resistance, i.e. N value, in bedrock.

Table 2 contains the recommended values for the nominal side and tip resistance for drilled shafts founded in the underlying competent bedrock. The upper three feet of competent bedrock penetration shall not be used for drilled shaft resistance due to the likelihood of construction disturbance and possible additional weathering. To account for axial group effects, the minimum spacing requirements between drilled shafts should be three diameters from center-to-center.

Table 2. Recommended Drilled Shaft Axial Resistance

Reference Boring	Approximate Top of Competent Bedrock Elevation (feet)	Tip Resistance (ksf)		Side Resistance, (ksf)	
		Nominal	Factored ($\Phi=0.5$)	Nominal	Factored ($\Phi=0.55$)
J-15-G B-1	7987.5	150	75	15	8.2
J-15-G B-2	7983.5	150	75	15	8.2

3.2.2 Drilled Shaft Lateral Resistance

The input parameters provided in Table 3 are recommended for use with the computer program LPILE to develop the soil models used to evaluate the drilled shaft response to lateral loading. Table 3 provides the estimated values associated with the soil types encountered in the borings. They can also be used for driven H-piles, which will be described in Section 3.3. The nature and type of loading should be considered carefully. Individual soil layers and their extent can be averaged or distinguished by referring to the boring logs at the locations of the proposed bridge. The soils and/or bedrock materials prone to future disturbance, such as from utility excavations or frost heave, should be neglected in the lateral load analyses to the depth of disturbance, which may require more than but should not be less than three feet.

Recommendations for p-y multiplier values (P_m values) to account for the reduction in lateral capacity due to group effects are provided in Section 10.7.3.12 of AASHTO (2020). The P_m value will depend on the direction of the applied load, center-to-center spacing, and location of the foundation element within the group.



Table 3. LPILE Parameters

Material Type	LPILE Soil Criteria	Effective Unit Weight (pcf)		Friction Angle, (deg.)	Unconfined Compressive Strength (psi)	Strain Factor, ε50	p-y modulus kstatic (pci)	
		AGT ¹	BGT ²				AGT ¹	BGT ²
Class 1 Structure Backfill	Sand (Reese)	130	67.5	34	-	-	90	60
Fill and Native Sandy Soils	Sand (Reese)	125	62.5	28	-	-	25	20
Granite/Diorite Bedrock	Strong Rock (Vuggy Limestone)	140	140	-	3,800	0.004	-	-

Note: ¹Above Groundwater Table
²Below Groundwater Table

3.2.3 General Drilled Shaft Recommendations

The following recommendations can be used in the design and construction of the drilled shafts.

- Groundwater and potentially caving soils may be encountered during drilling depending on the time of year and location. The Contractor shall construct the drilled shafts using means and methods that maintain a stable hole.
- Bedrock may be very hard at various elevations. The contractor should mobilize equipment of sufficient size and operating condition to achieve the required design bedrock penetration.
- Drilled shaft construction shall not disturb previously installed drilled shafts. The drilled shaft concrete should have sufficient time to cure before construction on a drilled shaft within three shaft diameters (center to center spacing) begins to prevent interaction between shafts during excavation and concrete placement.
- Based on the results of the field investigation and experience with similar properly constructed drilled shaft foundations, it is estimated that foundation settlement will be less than approximately ½ inch when designed according to the criteria presented in this report.
- A representative of the Contractor’s engineer should observe drilled shaft installation operations on a full-time basis.

3.3 Driven H-Pile Recommendations

3.3.1 Driven H-Pile Axial Resistance

Steel H-piles driven into bedrock may be designed for a nominal axial resistance equal to 34 kips per square inch (ksi) multiplied by the cross-sectional area of the pile for piles composed of Grade 50 ksi steel for use with LRFD Strength Limit State design. Piles should be driven to refusal into the underlying bedrock as defined in Section 502.05 of CDOT (2019). A wave equation analysis using the Contractor’s pile driving equipment is necessary to estimate pile drivability.

Based on the strength of the granite bedrock encountered during our investigation, it is likely that refusal will be met within the upper 1 to 2 feet of competent bedrock. Holes may need to be pre-drilled to meet the requirement for pile design tip elevations.



3.3.2 Driven H-Pile Axial Resistance Factors

Assuming a pile driving analyzer (PDA) is used to monitor pile driving per Section 502 of CDOT (2019), a resistance factor of 0.65 may be used per AASHTO (2020) Table 10.5.5.2.3-1. Section 502.05 of CDOT (2019) stipulates that if PDA is used, a minimum of one PDA monitoring per bridge bent be performed to determine the condition of the pile, efficiency of the hammer, static bearing resistance of the pile, and to establish pile driving criteria. Per AASHTO (2020) recommendations, a resistance factor of 0.5 can be used for wave equation analysis only without pile dynamic measurements such as PDA monitoring. Per AASHTO (2020) recommendations, a resistance factor of 0.75 may be used if a successful static load test is conducted per site condition.

3.3.3 Driven H-Pile Lateral Resistance

The information provided previously in Section 3.2.2 may be used to evaluate H-pile lateral resistance.

3.3.4 General Driven H-Pile Recommendations

The following recommendations are for the design and construction of driven H-piles.

1. Based on the results of the field exploration and our experience with similar properly constructed driven pile foundations, it is estimated that settlement will be less than approximately ½ inch when designed according to the criteria presented in this report.
2. A minimum spacing requirement for the piles should be three diameters (equivalent) center to center.
3. Driven piles should be driven with protective cast steel pile points or equivalent to provide better pile tip seating and to prevent potential damage from coarse soil particles, which may be present at the site.
4. A qualified representative of the Contractor's engineer should observe pile-driving activities on a full-time basis. Piles should be observed and checked for crimping, buckling, and alignment. A record should be kept of embedment depths and penetration resistances for each pile.
5. It is estimated that the piles will penetrate approximately 1 to 2 feet into competent bedrock (see Table 1 for the estimated elevation for the top of competent bedrock). The final tip elevations will depend on bedrock conditions encountered during driving.
6. If the pile penetration extends below the estimated pile penetration into bedrock by 10 feet or more, the pile driving operations should be temporarily suspended for dynamic monitoring with PDA. We recommend that the subject pile be allowed to rest overnight or longer before restriking and monitoring the beginning-of-restrike with a PDA. The data collected with the PDA shall then be reduced using the software CAPWAP to determine the final nominal pile resistance. The pile driving criteria may be modified by CDOT's or the Contractor's engineer based on the PDA/CAPWAP results.

3.4 CBC Foundation Recommendations

To assure adequate foundation support and to minimize the potential for differential settlement, we recommend that the exposed subgrade soils should be scarified a minimum of 6 inches, moisture conditioned, and re-compacted in accordance with Section 203.07 of the CDOT Standard Specifications (2019) before the placement of structural elements or structural backfill. If unsuitable or soft materials are encountered after the excavation, the materials may be removed and replaced with CDOT Class 1 Structure Backfill in accordance with Section 203.07 of the CDOT Standard Specifications (2019). Visual inspection of the foundation excavations should be performed by a qualified representative of the Geotechnical Engineer of record to identify the quality of the foundation materials prior to placement of backfill and the CBC. Groundwater may be encountered during excavation for the subgrade preparation. Groundwater control systems may be required to prevent seepage migrating into the construction zone by creating groundwater cut-off and/or dewatering systems.



The recommended nominal bearing resistance using Strength Limit State for the CBC and associated wing walls for both moist and saturated conditions are provided in Table 4. We assume the materials in contact with the bottom of the proposed CBC and wing walls will consist of native sand soils or CDOT Class 1 Structure Backfill placed in accordance with Section 203.07 of the CDOT Standard Specifications (2019). The reduced footing width due to eccentricity can be calculated based on the recommendations in Sections 11.6.3.2 and 11.10.5.4 of AASHTO (2020). A bearing resistance factor of 0.45 may be used for shallow foundations based on the recommendations in Table 10.5.5.2.2-1 of AASHTO (2020).

Table 4. Bearing Resistance for CBC and Wing Walls on Shallow Foundation

Soil Conditions	Nominal Bearing Resistance (ksf) ^{1, 2}
Moist	$1.94 + 1.0 * B'$
Saturated	$0.97 + 0.5 * B'$

¹ B' is the footing width in feet reduced for eccentricity (e). $B' = B - 2e$, where B is the nominal foundation width.
² The calculated nominal bearing resistance is based on a minimum 12 inches of embedment and shall be limited to 10 ksf.

The proposed CBC will be at the location of the existing CMP, and as needed, a portion of the CBC will be in a cut area, therefore it is estimated that the total settlement of the structure will be minimal and will occur during construction. The structure settlement is partially controlled by the weight of the adjacent embankment fill. Thus, it is recommended that the embankment fill on both sides of the CBC be placed at a relatively uniform elevation.

Resistance to sliding at the bottom of foundations can be calculated based on a coefficient of friction at the interface between the pre-cast concrete and the existing native soils or compacted CDOT Class 1 Structure Backfill. The recommended nominal coefficients of friction and the corresponding resistance factors for Class 1 Structure Backfill and native soils are provided in Table 5.

Table 5. Coefficients of Friction for CBC and Wing Walls on Shallow Foundation

Foundation Soil Type	Coefficient of Friction	Resistance Factor
Class 1 Structure Backfill	0.53	0.9
Native Silty Sand	0.31	0.8

Backfill adjacent to the CBC should be Class 1 Structure Backfill, compacted with moisture density control. Backfill materials shall have a Class 0 for severity of sulfate exposure. Fill should be tested for severity of sulfate exposure prior to acceptance.

The passive pressure against the sides of the foundation is typically ignored; however, passive resistance can be used if long-term protection from disturbance, such as frost heave, future excavations, etc., is assured. Table 6 presents recommendations for the passive soil resistances for the encountered soil conditions. The passive resistance estimates are calculated from Figure 3.11.5.4-1 in AASHTO (2020) where a portion of the slip surface is modeled as a logarithmic spiral, the backslope is horizontal and the passive soil/concrete interface friction angle is equal to 60 percent of the soil's friction angle.



The recommended passive earth pressure resistances are presented in terms of an equivalent fluid unit weight for moist and saturated conditions. The recommended passive earth pressure values assume mobilization of the nominal soil/concrete foundation interface shear strength. A suitable resistance factor should be included in the design to limit the strain, which will occur at the nominal shear strength, particularly in the case of passive resistance. The resultant passive earth force, calculated from the equivalent fluid unit weight, should be applied at a point located 1/3 of the height of the soil (in contact with the foundation) above the base of the foundation, directed upward at an angle of 20 degrees from the horizontal.

Table 6. Passive Soil Resistance for CBC

Passive Soil Resistance	Soil Type	Nominal Resistance	Resistance Factor
	Moist	346 psf/ft	0.50
	Saturated	173 psf/ft	0.50

3.5 Lateral Earth Pressures

External loads used in the analyses of the bridge abutments and CBC wing walls should include earth pressure loads, traffic loads, and any other potential surcharge loads. Typical drainage details consisting of inlets near the abutments, geocomposite strip drains, and perforated pipes shall be included in the design to properly contain and transfer surface and subsurface water without saturating the soil around the abutments.

All abutment and CBC wing wall backfill materials should meet the requirements for CDOT Structure Backfill Class 1 in accordance with CDOT (2019). All backfill adjacent to the abutments and walls shall be placed and compacted in accordance with CDOT (2019). It is recommended that compaction of backfill materials be observed and evaluated by an experienced Contractor’s engineer or Contractor’s engineer’s representative.

A lateral wall movement or rotation of approximately 0.1 to 0.2 percent of the wall height may be required to mobilize active earth pressure for the recommended backfill materials. If the estimated wall movement is less than this amount, an at-rest soil pressure should be used in design. In order to mobilize passive earth pressure, lateral wall movement or rotation of approximately 1.0 to 2.0 percent of the wall height may be required for the recommended backfill materials. It should be carefully considered if this amount of movement can be accepted before passive earth pressure is used in the design.

Earth pressure loading within and along the back of the bridge abutments and CBC wing walls shall be controlled by the structural backfill. We recommend that active, at-rest, and passive lateral earth pressures used for the design of the structures be based on an effective angle of internal friction of 34 degrees, and a unit weight of 135 pounds per cubic foot (pcf) for CDOT Structure Backfill Class 1. The following can be used for design assuming a horizontal backslope:

- Active earth pressure coefficient (k_a) of 0.28
- Passive earth pressure coefficient (k_p) of 3.53
- At-rest earth pressure coefficient (k_0) of 0.44

Lateral earth pressures for a non-horizontal backslope can be estimated using section 3.11 in AASHTO (2020).



3.6 Bridge Scour Parameters

A bulk sample of the channel bed soils/rock near the existing structure was collected for gradation analysis. The results of the grain size analysis are presented in Appendix C.

4 BRIDGE APPROACH PAVEMENT

Pavement borings were located approximately 250 feet beyond the existing CMP on each side. Prior to drilling, the existing pavement was cored with a 4-inch nominal diameter core barrel. Photos of the pavement core, logs of the subsurface soils/rock, and results of geotechnical and analytical laboratory testing are presented in the appendices. Bulk soil samples were collected from the pavement borings and combined for classification, strength (R-value), and analytical testing. Preliminary pavement thickness design will be completed by CDOT Staff materials. The asphalt pavement thicknesses, aggregate base thicknesses (if present), subgrade soil classifications, and subgrade R-values are presented in Table 7.

Table 7. Existing Pavement Section and Subgrade Properties

Boring ID	Existing Asphalt Concrete Thickness (in)	Aggregate Base Thickness (in)	Subgrade Soil Classification (AASHTO) ¹	R-Value ¹
J-15-G P-1	6.0	Not Encountered	A-1-b (0)	19
J-15-G P-2	7.5	Not Encountered		

Note: ¹ Subgrade Classification and R-value test results based on combined bulk sample from each pavement boring

5 ANALYTICAL TEST RESULTS

Analytical testing was completed on representative samples of soils encountered in the borings. The test results can be found in Appendix C and are summarized in Table 8. The Analytical results should be used to select the proper concrete type for the project in accordance with CDOT Standard Specifications (2019). A qualified corrosion engineer should review the laboratory data and boring logs to determine the appropriate level of corrosion protection for materials in contact with these soils.

Table 8. Analytical Test Results

Sample Boring ID	Material	Water Soluble Sulfates, %	Water Soluble Chlorides, %	pH	Resistivity, ohm-cm
J-15-G P-1/P-2	Silty Sand (Fill)	0.008	0.0533	-	-
J-15-G B-1	Silty Sand	0.003	0.0024	7.6	5081
J-15-G B-2	Silty Sand	0.002	0.0013	7.7	6748

6 SEISMIC CONSIDERATIONS

No active faults are known to exist in the immediate vicinity of the proposed structure location. Based on the site class definitions provided in Table 3.10.3.1-1 of AASHTO LRFD (2020), the site can be categorized as Site Class D. Also based on the recommendations in Table 3.10.6-1 of AASHTO LRFD (2020), the bridge site can be classified as Seismic Zone 1.



The peak ground acceleration (PGA) and the short- and long- period spectral acceleration coefficients (S_s and S_1 , respectively) for Site Class B (reference site class) were determined using the seismic design maps from the USGS website. The seismic design parameters for Site Class D are shown in Table 9.

Table 9. Seismic Design Parameters

PGA (0.0 sec)	S_s (0.2 sec)	S_1 (1.0 sec)
0.072 g	0.152 g	0.043 g
A_s (0.0 sec)	S_{DS} (0.2 sec)	S_{D1} (1.0 sec)
0.115 g	0.243 g	0.103 g

7 LIMITATIONS

Our scope of services was performed, and this report was prepared in accordance with generally accepted principles and practices in this area at the time this report was prepared. We make no other warranty, either express or implied.

The classifications, conclusions, and recommendations submitted in this report are based on the data obtained from published and unpublished maps, reports, and geotechnical analyses. Our conclusions and recommendations are based on our understanding of the project as described in this report and the site conditions as interpreted from the explorations. This data may not necessarily reflect variations in the subsurface conditions and water levels occurring at other locations.

The nature and extent of subsurface variations may not become evident until excavation is performed. Variations in the data may also occur with the passage of time. If during construction, fill, soil, rock, or groundwater conditions appear to be different from those described in this report, this office should be advised immediately so we could review these conditions and reconsider our recommendations. If there is a substantial lapse of time between the submission of this report and the start of work at the site, or if conditions have changed because of natural forces or construction operations at or adjacent to the site, we recommend that this report be reviewed to determine the applicability of the conclusions and recommendations concerning the changed conditions or time lapse. We recommend on-site observation of foundation excavations and foundation subgrade conditions by an experienced geotechnical engineer or engineer's representative.

The scope of services of this study did not include hazardous materials sampling or environmental sampling, investigation, or analyses. In addition, we did not evaluate the site for potential impacts to natural resources, including wetlands, endangered species, or environmentally critical areas.

8 REFERENCES

AASHTO LRFD, 9th Edition. AASHTO Load Resistance Factor Design (LRFD) Bridge Design Specifications, Eight Edition. Washington, DC: American Association of State Highway and Transportation Officials. 2020.

Abu-Hejleh, N., O'Neill, M.W., Hanneman, Dennis, Atwooll, W.J., 2003. Improvement of the Geotechnical Axial Design Methodology for Colorado's Drilled Shafts Socketed in Weak Rocks, Final Report: Colorado Department of Transportation Research Branch, July 2003, Report No. CDOT-DTD-R-2003-6.

Colorado Department of Transportation, 2019. CDOT Standard Specifications for Road and Bridge Construction. 2019 Edition.

Respectfully Submitted,
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Attachments:

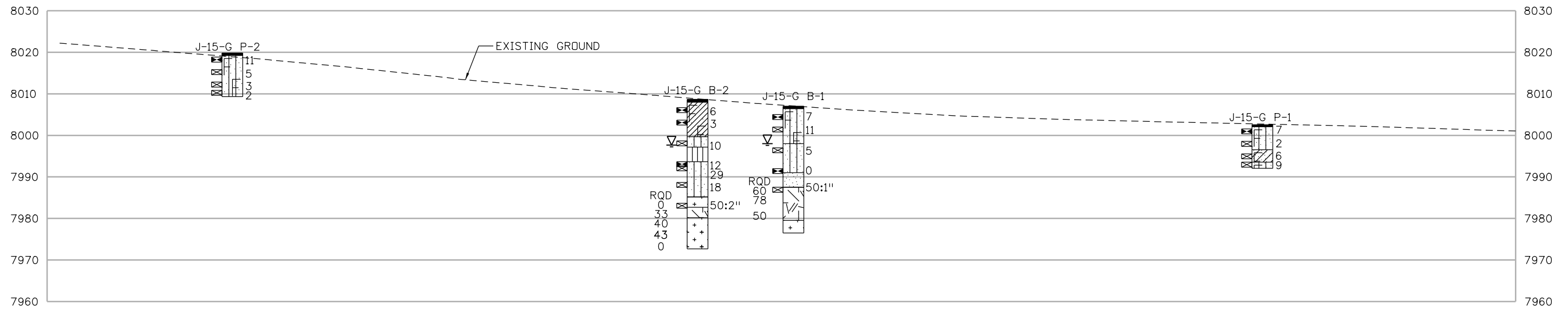
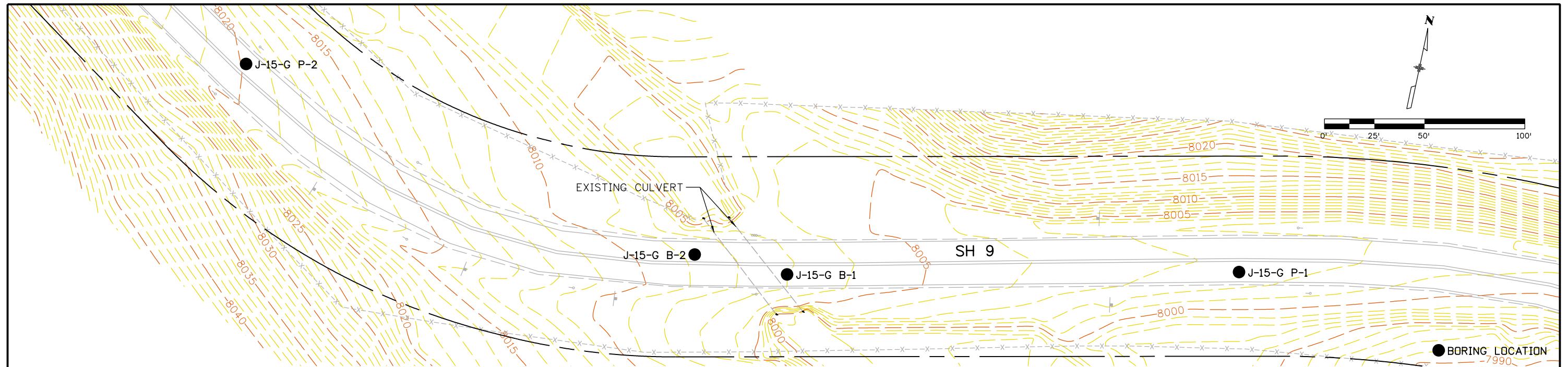
- Appendix A
- Appendix B
- Appendix C

APPENDIX A

ENGINEERING GEOLOGY SHEET

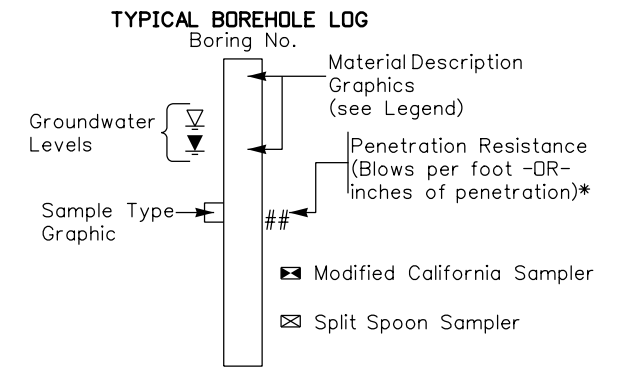


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LEGEND

	Asphalt		USCS Silty Sand		Fill
	USCS Poorly-graded Sand		Low Plasticity Sandy Clay		USCS Silt
	USCS Clayey Sand		Diorite		Granite



*e.g. A value of 50/3 or 50:3" indicates that 50 blows were applied to the sampler, with a penetration of 3 inches.

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File Name: 23558GEOE\Engineering Geology J-15-G.dgn		<p>Horiz. Scale: 1:50 Vert. Scale: As Noted</p> <p>Unit Information Unit Leader Initials</p>		<p>Yeh and Associates, Inc. Geotechnical - Geological - Construction Services</p>																						

APPENDIX B

KEY TO BORING LOGS

BORING LOGS

PAVEMENT CORE PHOTOS

ROCK CORE PHOTOS



Legend for Symbols Used on Borehole Logs

Sample Types



Bulk Sample of auger/odex cuttings



Rock core



Modified California Sampler (2.5 inch OD, 2.0 inch ID)



Standard Penetration Test (ASTM D1586)

Drilling Methods



CORING



HOLLOW-STEM AUGER

Lithology Symbols (see Boring Logs for complete descriptions)



Asphalt



Cobbles and gravel



USCS Fat/High Plasticity Clay



USCS Lean/Low Plasticity Clay



Fill



Fill with Clay as major soil



Fill with Gravel as major soil



USCS Clayey Gravel



USCS Silty, Clayey Gravel



USCS Poorly-graded Gravel



USCS Poorly-graded Gravel with Clay



Low Plasticity Gravelly Clay



USCS Silt



USCS Low Plasticity Organic silt or clay



High Plasticity Sandy Clay



Poorly-graded Sandy Gravel



Low Plasticity Sandy Clay



USCS Clayey Sand



USCS Silty Sand



USCS Poorly-graded Sand



Cobbles and gravel



Diorite



Gneiss



Granite



Limestone



Sandstone



Shale



Weathered Bedrock

Lab Test Standards

Moisture Content	ASTM D2216
Dry Density	ASTM D7263
Sand/Fines Content	ASTM D421, ASTM C136, ASTM D1140
Atterberg Limits	ASTM D4318
AASHTO Class.	AASHTO M145, ASTM D3282
USCS Class.	ASTM D2487
(Fines = % Passing #200 Sieve)	
Sand = % Passing #4 Sieve, but not passing #200 Sieve)	

Other Lab Test Abbreviations

pH	Soil pH (AASHTO T289-91)
S	Water-Soluble Sulfate Content (AASHTO T290-91, ASTM D4327)
Chl	Water-Soluble Chloride Content (AASHTO T291-91, ASTM D4327)
S/C	Swell/Collapse (ASTM D4546)
UCCS	Unconfined Compressive Strength (Soil - ASTM D2166, Rock - ASTM D7012)
R-Value	Resistance R-Value (ASTM D2844)
DS (C)	Direct Shear cohesion (ASTM D3080)
DS (phi)	Direct Shear friction angle (ASTM D3080)
Re	Electrical Resistivity (AASHTO T288-91)
PtL	Point Load Strength Index (ASTM D5731)

Notes

1. Visual classifications are in general accordance with ASTM D2488, "Standard Practice for Description and Identification of Soils (Visual-Manual Procedures)".
2. "Penetration Resistance" on the Boring Logs refers to the uncorrected N value for SPT samples only, as per ASTM D1586. For samples obtained with a Modified California (MC) sampler, drive depth is 12 inches, and "Penetration Resistance" refers to the sum of all blows. Where blow counts were > 50 for the 3rd increment (SPT) or 2nd increment (MC), "Penetration Resistance" combines the last and 2nd-to-last blows and lengths; for other increments with > 50 blows, the blows for the last increment are reported.
3. The Modified California sampler used to obtain samples is a 2.5-inch OD, 2.0-inch ID (1.95-inch ID with liners), split-barrel sampler with internal liners, as per ASTM D3550. Sampler is driven with a 140-pound hammer, dropped 30 inches per blow.
4. "ER" for the hammer is the Reported Calibrated Energy Transfer Ratio for that specific hammer, as provided by the drilling company.



Boring Began: 10/1/2020
Boring Completed: 10/1/2020
Drilling Method(s): Hollow-Stem Auger
Driller: Vine Laboratories
Drill Rig: CME 750X Buggy
Hammer: Automatic (hydraulic), ER: 80%

Total Depth: 10.5 ft
Ground Elevation: 8002.5
Coordinates: N: 305376.9 E: 1005897.4
Location: State Highway 9, southbound outside lane
Logged By: C. Wallace
Final By: J. McCall

Weather Notes: Clear, 40s
Inclination from Horiz.: Vertical
Night Work:

Groundwater Levels: Not Observed			
Symbol	-	-	-
Depth	-	-	-
Date	-	-	-

BORING LOG 2019 - SPT CDOT STYLE 220-063 R2 BRIDGE BUNDLE FIXED FORMATTING 12-11-2020.GPJ 2019 YEH COLORADO TEMPLATE.GDT 2019 YEH COLORADO LIBRARY.GLB 12/17/20

Elevation (feet)	Depth (feet)	Sample Type/Depth	Drilling Method	Soil Samples		Lithology	Material Description	Moisture Content (%)	Dry Density (pcf)	Gravel Content (%)	Sand Content (%)	Fines Content (%)	Atterberg Limits		AASHTO & USCS Classifications	Field Notes and Other Lab Tests
				Blows per 6 in	Penetration Resistance								Liquid Limit	Plasticity Index		
8000	0.0 - 0.5	ASPHALT (6 inches)														
8000	0.5 - 6.0	Silty SAND (SM) (Fill), dark brown, moist, loose, micaceous.		3-4	7											
7995	5			1-1-1	2		10.1		6.0	62.8	31.2	28	2	A-2-4 (0) SM		
7995	6.0 - 9.0	Clayey SAND (SC) (Fill), brown, moist, loose.		2-2-4	6											
7980	9.0 - 10.5	Silty SAND with gravel (SM) (Fill), brown, moist, loose.		3-4-5	9											
Bottom of Hole at 10.5 ft.																



Boring:	P-1	AC:	6"
Roadway:	State Highway 9	PCC:	-
Direction:	Southbound	Base:	-
Lane:	Outside	Notes:	-



Boring:	P-2	AC:	7.5"
Roadway:	State Highway 9	PCC:	-
Direction:	Northbound	Base:	-
Lane:	Outside	Notes:	-



Yeh and Associates, Inc.
Geotechnical • Geological • Construction Services

Pavement Core Photographs

FIGURE

PROJECT NO. 220-063 DATE: 12/14/2020
 FIGURE BY: BHL YEH OFFICE: Colorado Springs
 CHECKED BY: JTM

CDOT Region 2 Bridge Bundle
Structure J-15-G

B-1



Yeh and Associates, Inc.
 Geotechnical • Geological • Construction Services

PROJECT NO. 220-063 DATE: 12/14/2020
 FIGURE BY: BHL YEH OFFICE: Colorado Springs
 CHECKED BY: JTM

Rock Core Photos
Boring: B-1
Depth: 19.5' - 29.5'

CDOT Region 2 Bridge Bundle
 Structure J-15-G

FIGURE

B-2




Yeh and Associates, Inc.
 Geotechnical • Geological • Construction Services

PROJECT NO. 220-063 DATE: 12/14/2020
 FIGURE BY: BHL YEH OFFICE: Colorado Springs
 CHECKED BY: JTM

Rock Core Photos
Boring: B-1
Depth: 29.5' - 30.5'

CDOT Region 2 Bridge Bundle
 Structure J-15-G

FIGURE

B-3



Yeh and Associates, Inc.

Geotechnical • Geological • Construction Services

PROJECT NO.	220-063	DATE:	12/14/2020
FIGURE BY:	BHL	YEH OFFICE:	Colorado Springs
CHECKED BY:	JTM		

Rock Core Photos
Boring: B-2
Depth: 24.5' - 34.5'

CDOT Region 2 Bridge Bundle
 Structure J-15-G

FIGURE

B-4




Yeh and Associates, Inc.
 Geotechnical • Geological • Construction Services

PROJECT NO. 220-063 DATE: 12/14/2020
 FIGURE BY: BHL YEH OFFICE: Colorado Springs
 CHECKED BY: JTM

Rock Core Photos
Boring: B-2
Depth: 34.5' - 36'

CDOT Region 2 Bridge Bundle
 Structure J-15-G

FIGURE

B-5

APPENDIX C

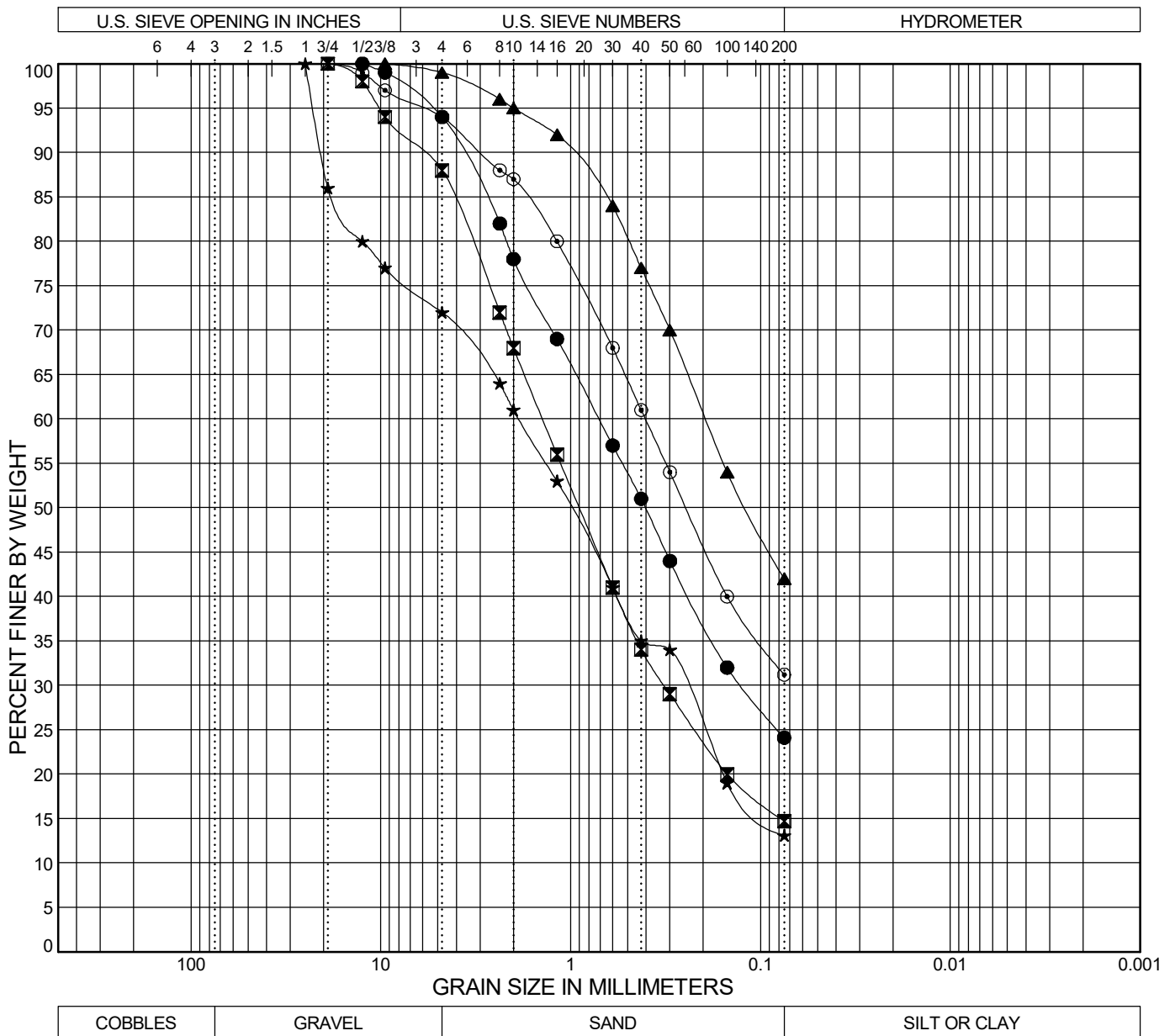
SUMMARY OF LABORATORY TEST RESULTS



Summary of Laboratory Test Results

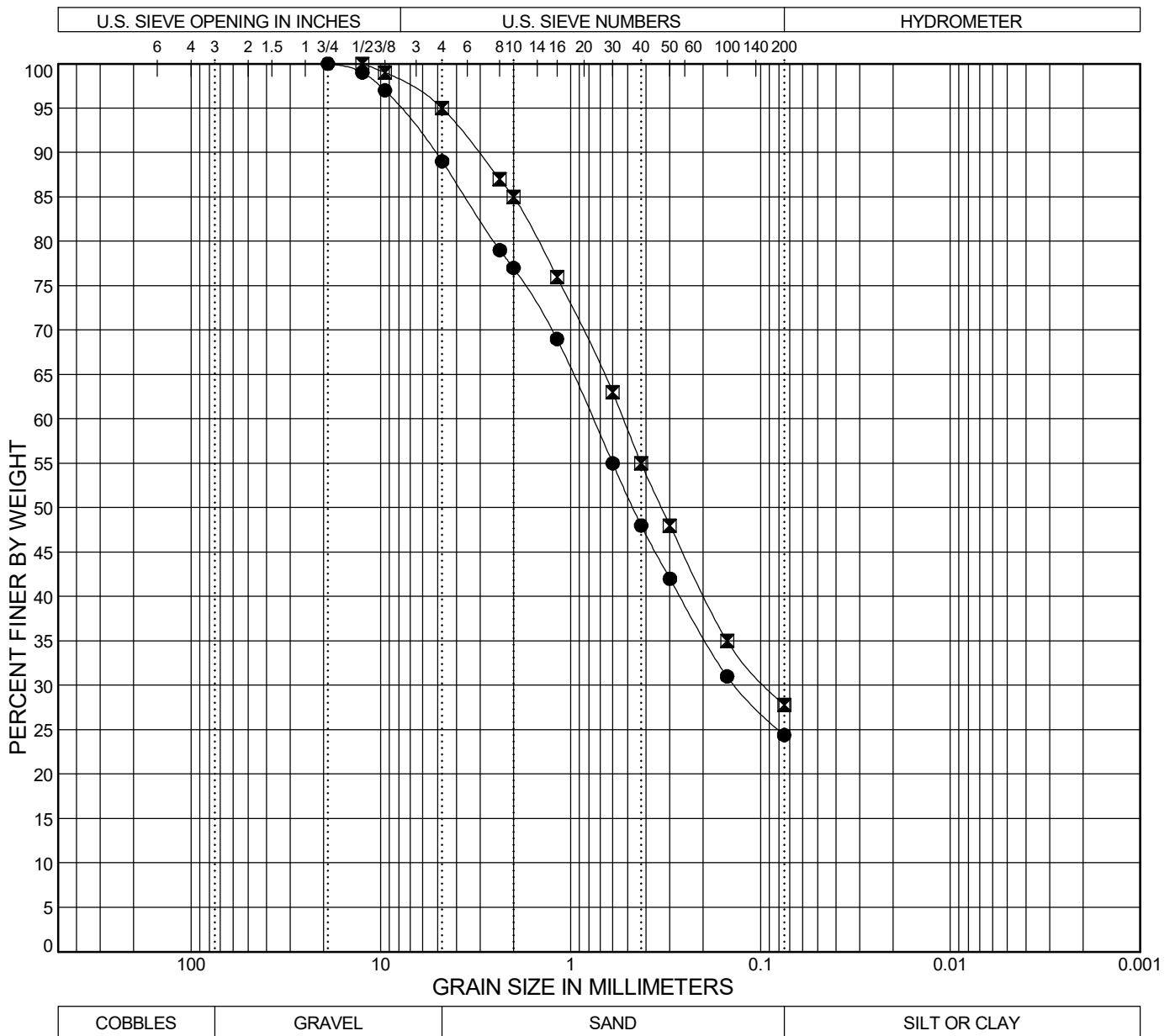
Project No: 220-063 Project Name: CDOT Region 2 Bridge Bundle Date: 12-17-2020

Sample Location			Natural Moisture Content (%)	Natural Dry Density (pcf)	Gradation			Atterberg			pH	Water Soluble Sulfate (%)	Water Soluble Chloride (%)	Resistivity (ohm-cm)	Swell (+) / Collapse (-) (% at Load in psf)	Unconf. Comp. Strength (psi)	R-Value	Classification	
Boring No.	Depth (ft)	Sample Type			Gravel > #4 (%)	Sand (%)	Fines < #200 (%)	LL	PL	PI								AASHTO	USCS
J-15-G B-1	2.0	MC	8.2		6.0	69.9	24.1	26	23	3								A-2-4 (0)	SM
J-15-G B-1	10.0	SPT	15.2		12.0	73.3	14.7	NV	NP	NP	7.6	0.003	0.0024	5081				A-1-b (0)	SM
J-15-G B-1	22.0	CORE													12740				
J-15-G B-2	5.0	MC	21.6	95.1	1.0	57.0	42.0	33	25	8								A-4 (1)	SM
J-15-G B-2	20.0	SPT	11.9		28.0	58.9	13.1	26	22	4	7.7	0.002	0.0013	6748				A-1-b (0)	SM
J-15-G B-2	27.0	CORE													11410				
J-15-G P-1	4.0	SPT	10.1		6.0	62.8	31.2	28	26	2								A-2-4 (0)	SM
J-15-G P-1/P-2	2.5	BULK	7.1		11.0	64.6	24.4	26	22	4		0.008	0.0533			19		A-1-b (0)	SM
J-15-G P-2	1.0	MC	9		5.0	67.2	27.8	26	23	3								A-2-4 (0)	SM
J-15-G Scour	0	BULK	5.4		13.0	79.7	7.3	NV	NP	NP								A-1-b (0)	SP-SM




BOREHOLE	DEPTH (ft)	AASHTO Classification	USCS Classification	LL	PL	PI	%Gravel	%Sand	%Fines	
									%Silt	%Clay
● J-15-G B-1	2.0	A-2-4 (0)	SM	26	23	3	6.0	69.9	24.1	
⊠ J-15-G B-1	10.0	A-1-b (0)	SM	NV	NP	NP	12.0	73.3	14.7	
▲ J-15-G B-2	5.0	A-4 (1)	SM	33	25	8	1.0	57.0	42.0	
★ J-15-G B-2	20.0	A-1-b (0)	SM	26	22	4	28.0	58.9	13.1	
◎ J-15-G P-1	4.0	A-2-4 (0)	SM	28	26	2	6.0	62.8	31.2	

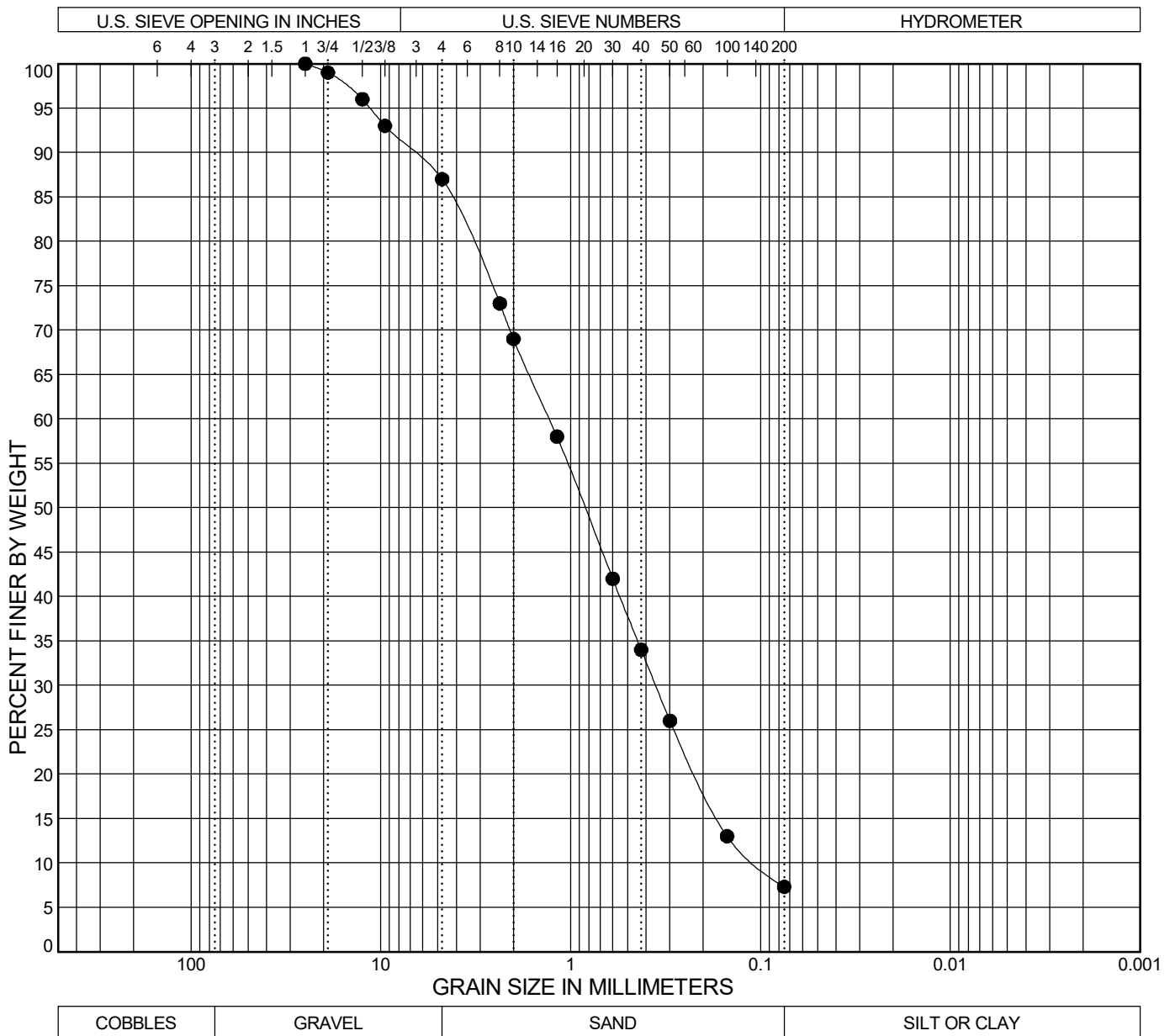
 Yeh and Associates, Inc. Geotechnical • Geological • Construction Services	SIEVE ANALYSIS		FIGURE C-1
	Project No. 220-063 Date: 12-17-2020 Report By: D. Gruenwald Yeh Lab: Colorado Springs Checked By: J. McCall	CDOT Region 2 Bridge Bundle Structure J-15-G	




BOREHOLE	DEPTH (ft)	AASHTO Classification	USCS Classification	LL	PL	PI	%Gravel	%Sand	%Fines	
									%Silt	%Clay
● J-15-G P-1/P-2	2.5	A-1-b (0)	SM	26	22	4	11.0	64.6	24.4	
☒ J-15-G P-2	1.0	A-2-4 (0)	SM	26	23	3	5.0	67.2	27.8	

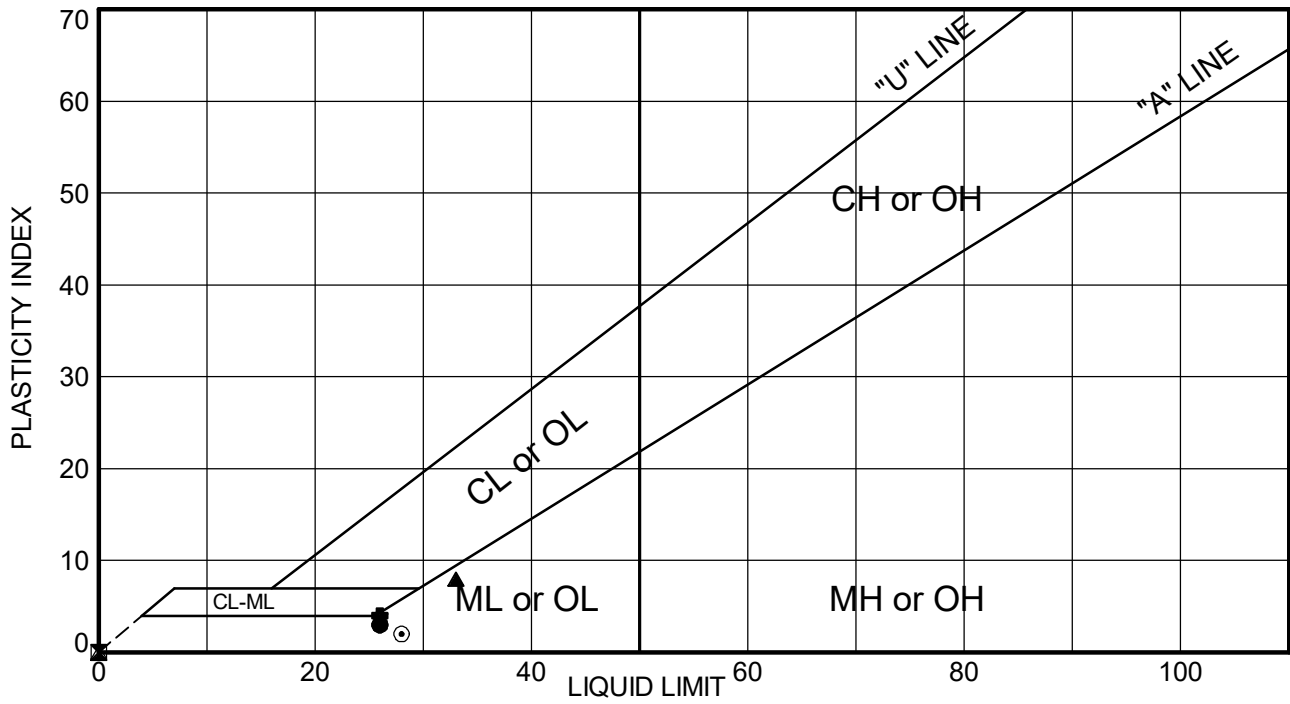
 Yeh and Associates, Inc. Geotechnical • Geological • Construction Services	<h2>SIEVE ANALYSIS</h2>	<h2>FIGURE</h2>

03 GRAIN SIZE YEH 220-063 R2 BRIDGE BUNDLE FIXED FORMATTING 12-11-2020.GPJ 2019 YEH COLORADO TEMPLATE.GDT 2019 YEH COLORADO LIBRARY.GLB 12/17/20



BOREHOLE	DEPTH (ft)	AASHTO Classification	USCS Classification	LL	PL	PI	%Gravel	%Sand	%Fines	
									%Silt	%Clay
● J-15-G Scour	0.0	A-1-b (0)	SP-SM	NV	NP	NP	13.0	79.7	7.3	

 Yeh and Associates, Inc. Geotechnical • Geological • Construction Services	SIEVE ANALYSIS		FIGURE C- 3
	Project No. 220-063 Date: 12-17-2020 Report By: D. Gruenwald Yeh Lab: Colorado Springs Checked By: J. McCall	CDOT Region 2 Bridge Bundle Structure J-15-G	



BOREHOLE	DEPTH (ft)	LL	PL	PI	Passing #200	USCS Sample Description and Symbol	AASHTO Class.
● J-15-G B-1	2.0	26	23	3	24.1	SILTY SAND (SM)	A-2-4 (0)
☒ J-15-G B-1	10.0	NV	NP	NP	14.7	SILTY SAND (SM)	A-1-b (0)
▲ J-15-G B-2	5.0	33	25	8	42.0	SILTY SAND (SM)	A-4 (1)
★ J-15-G B-2	20.0	26	22	4	13.1	SILTY SAND with GRAVEL (SM)	A-1-b (0)
⊙ J-15-G P-1	4.0	28	26	2	31.2	SILTY SAND (SM)	A-2-4 (0)
⊕ J-15-G P-1/P-2	2.5	26	22	4	24.4	SILTY SAND (SM)	A-1-b (0)
○ J-15-G P-2	1.0	26	23	3	27.8	SILTY SAND (SM)	A-2-4 (0)
△ J-15-G Scour	0.0	NV	NP	NP	7.3	POORLY GRADED SAND with SILT (SP-SM)	A-1-b (0)



ATTERBERG LIMITS

FIGURE

Project No. 220-063 Date: 12-17-2020
 Report By: D. Gruenwald Yeh Lab: Colorado Springs
 Checked By: J. McCall

CDOT Region 2 Bridge Bundle
 Structure J-15-G

C - 4



R Value ASTM D2844

CLIENT	Yeh & Associates	BORING NO.	J-15_g
JOB NO.	2546-128	DEPTH	Scour
PROJECT	--	SAMPLE NO.	BULK
PROJECT NO.	220-063	DATE SAMPLED	--
LOCATION	--	SAMPLED BY	--
DATE TESTED	11/23/20	DESCRIPTION	--
TECHNICIAN	ALH		

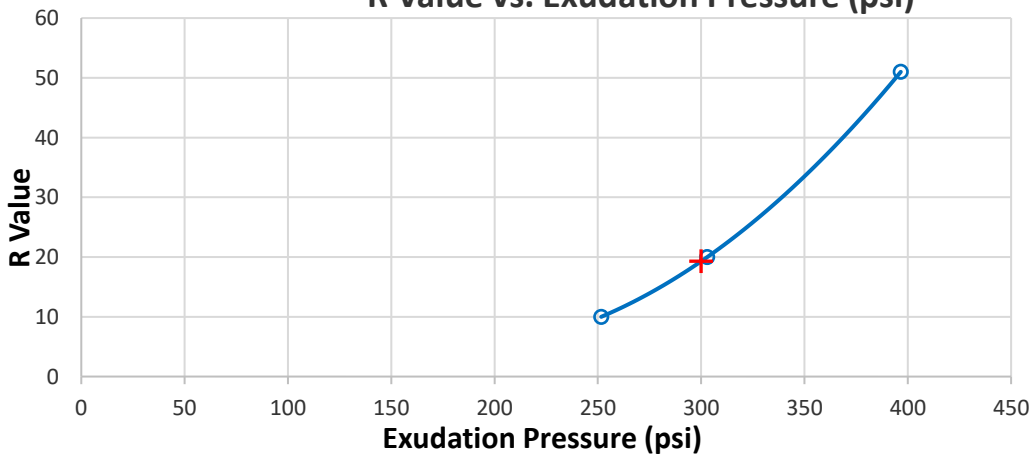
Sample Conditions

Mass of Wet Soil & Pan (g):	1219.8	1202.8	2007.8
Mass of Dry Soil & Pan (g):	1107.0	1103.9	1904.6
Mass of Pan (g):	14.3	14.1	843.1
Mass of Wet Soil & Mold (g):	3320.4	3293.3	3265.8
Mass of Mold (g):	2114.3	2101.0	2096.3
Sample Height (in):	2.60	2.56	2.51
Wet Density (pcf):	140.6	141.2	141.3
Dry Density (pcf):	127.5	129.4	128.7
Wet Density (kg/m ³):	2253	2262	2263
Dry Density (kg/m ³):	2042	2073	2062
Moisture (%):	10.3	9.1	9.7

R Value Data

Exudation Pressure (lbs):	3162	4984	3807
Exudation Pressure (psi):	251.6	396.6	303.0
2000 lbs. Dial Reading (psi):	130	57	103
Displacement Turns:	5.94	4.88	5.65
Uncorrected R Value:	9	48	20
Corrected R Value:	10	51	20

R Value vs. Exudation Pressure (psi)



Corrected R Value at 300 psi
Exudation Pressure

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NOTES:

Data entry by: ALH
 Checked by: KR
 File name: 2546128_R Value ASTM D2844_4.xlsm

Date: 11/24/20
 Date: 11/25/20